ERECTION NOTES

- 1. All bracing shown and provided by the Metal Building Provider (MBP) for this building is required and shall be installed by the erector
- as a permanent part of the structure ("Code of Standard Practice for Steel Buildings" in the ANSI/AISC 303-16; Section 7.10). 2. Temporary supports, such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined and furnished by the erector ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.10.3).
- Normal erection operations include the correction of minor misfits by moderate amounts of reaming, grinding, welding or cutting, and the drawing of elements into line through use of drift pins. Errors which require major changes in the member configuration are to be reported immediately to the Metal Building Provider by the customer to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others ("Code of Standard Practice for Steel Buildings and Bridges "in the ANSI/AISC 303-16; Section 7.14).
- 4. Erection tolerances are set forth in the "Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.13 note that individual members are considered plump, level and aligned if the deviation does not exceed 1:500. Variations in finished overall dimensions of structure steel framing are deemed within the limits of good practice when they do not exceed the cumulative effect of rolling, fabricating, and erection tolerances.
- 4.1. When crane support systems are part of the metal building system erection tolerances Section 6.8, Erection Tolerances, 2018 MBMA Metal Building Systems manual shall apply. To achieve the required tolerances grouting of the columns and shimming of the runway beams may be required. The customer shall provide grout if required. The contractor erecting the runway beams is responsible for shimming, plumbing, and leveling of the runway system. When aligning the runway beams the alignment shall be with respect to the beam webs so that the center of the aligned rail is over the runway web.
- 5. As a general rule field welding is not used to assemble a metal building system. In cases where the drawings indicate field welding and in cases where approved corrections are to be made by field welding the following requirements shall be met;
- welders must be qualified by an independent testing agency, with suitable documentation to AWS D1.1 Structural Welding Code Steel or AWS D1.3 Structural Welding Code — Sheet as applicable, for the processes, positions, and materials involved.
- All welds must be made in conformance to a documented and approved Welding Procedure Specification (WPS). All joints which are not pregualified must be supported by a certified Procedure Qualification Record (PQR) by an independent testing agency.
- 6. All documentation and records shall be the responsibility of the customer.
- 7. Any claims or shortages by buyer must be made to the Metal Building Provider within seven (7) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed. All claims should be directed to the Metal Building Provider's Customer Service Department.
- 8. Claims for correction of alleged misfits will be disallowed unless the Metal Building Provider shall have received prior notice thereof and allowed reasonable inspection of such misfits. Ordinary inaccuracies of shop work shall not be construed as misfits. No part of the building may be returned or charges assessed for alleged misfits without prior approval from the Metal Building Provider.
- Neither the Metal Building Provider nor the customer will cut, drill or otherwise after their work, or the work of other trades to accommodate other trades unless such work is clearly specified in the contract documents. Whenever such work is specified the customer is responsible for furnishing complete information as to materials, size, location, and number of alterations prior to preparation of shop drawings ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16, Section 7.15). 10. The Metal Building Provider Field Modifications Policy:
- 10.1. The Metal Building Provider will only be responsible for the field-modified parts designed and approved by the Metal Building Provider's Customer Service Department.
- 10.2. Any field modifications designed by third parties may not be approved by the Metal Building Provider and may limit the Metal Building Provider's warranty and liability.
- 10.3. The Metal Building Provider makes no warranty and hereby disclaims any responsibility with respect to the design, engineering, or
- construction of any field-modified parts performed by third parties. WARNING - SOME PANELS AND TRIM PARTS ARE FURNISHED WITH A PROTECTIVE PEEL-OFF FILM. PARTS PROVIDED WITH THIS FILM CANNOT BE EXPOSED TO SUNLIGHT WITHOUT FIRST REMOVING THE FILM. THIS FILM MUST BE REMOVED PRIOR TO INSTALLATION. FILM MUST ALSO BE REMOVED FROM ALL NON EXPOSED PARTS WITHIN SIX MONTHS FROM FILM APPLICATION OR IRREPARABLE DAMAGE WILL OCCUR TO THE SURFACE CLAIMS WILL NOT BE ACCEPTED FOR THIS ISSUE.

RESPONSIBILITIES

- 1. The Metal Building Provider Customer, hereafter referred to as the "customer," obtains and pays for all building permits, licenses public assessments, paving or utility pro rata, utility connections, occupancy fees and other fees required by any governmental authority or utility in connection with the work provided for in the Contract Documents. The customer provides at his expense all plans and specifications required to obtain a building permit. it is the customer's responsibility to ensure that all plans and specifications comply with the applicable requirements of any governing building authorities.
- The customer is responsible for identifying all applicable building codes, zoning codes, or other regulations applicable to the Construction Project, including the Metal Building system.
- 3. It is the responsibility of the customer to interpret all aspects of the End User's specifications and incorporate the appropriate specifications, design criteria, and design loads into the Order Documents submitted to the Metal Building Provider.
- 4. It is the responsibility of the Metal Building Provider to furnish the metal building system to meet the specifications including the design criteria and design loads incorporated by the Contractor into the Order Documents. The Metal Building Provider is not responsible for making an independent determination of any local codes or any other requirements not part of the Order Document.
- The Metal Building Provider's standard specifications apply unless stipulated otherwise in the Contract Documents. The Metal Building Provider design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work any other interpretations to the contrary not with standing, it is understood by both parties that the customer is responsible for clarifications of inclusions or exclusions from the Architectural plans.
- 6. In case of discrepancies between the Metal Building Provider's structural steel plans and plans for other trades, the Metal Building Provider's shall govern ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303-16; Section 3.3).
- The customer is responsible for overall project coordination. All interface, compatibility and design considerations concerning any materials not furnished by the Metal Building Provider and the Metal Building Provider's steel system are to be considered and coordinated by the customer. Specific design criteria concerning this interface between materials must be furnished by the customer before release for fabrication or the Metal Building Provider's assumptions will govern.
- 8. Foundations, anchor rods, and anchor rod embedment are designed, furnished, and set by the customer in accordance with an approved drawing. Dimensional accuracy shall satisfy the requirements of Section 7.5 1 of "Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303-16.
- 9. All other embedded items or connection materials between the structural steel and the work of other trades are located and set by the customer in accordance with approved location on erection drawings. Accuracy of these items must satisfy the erection tolerance
- 10. The Metal Building Provider does not investigate the influence of the metal building system on existing buildings or structures. The End Customer assures that such buildings and structures are adequate to resist snow drifts, wind loads, or other conditions as a result of the presence of the metal building system.

GENERAL SPECIFICATIONS

- 1. Wall and liner panels are an integral part of the structural system. Unauthorized removal of panels or cutting panels for framed openings not shown is prohibited
- Oil-canning, a perceived waviness inherent to light gauge metal, may exist. This condition does not affect the structural integrity or the finish of the panel, and therefor is not a cause for rejection.
- 3. The Metal Building Provider's red-oxide and gray-oxide primer are designed for short term field protection from exposure to ordinary atmospheric conditions. Primed steel which is stored in the field pending erection should be kept free of the ground, and so positioned as to minimize water—holding pockets, dust, mud, and other contamination of the primer film. Repairs of damage to primed surfaces and/or removal of foreign material due to transportation (e.g. road salt, de—icing chemicals and other substances encountered during transportation that may accelerate deterioration of the primer or corrosion of the underlying steel), improper field storage, or site conditions are not the responsibility of the Metal Building Provider. (MBMA, 2018 MBSM, Section 4.2.4)
- 4. All bolts are 1/2" x 1-1/4" A307 unless noted. Refer to the erection drawings for specific framing connections and the cross-section(s) for main frame connections.
- 5. Unless noted otherwise on the frame cross section(s), all bolted joints with ASTM F3125 Grade A325 bolts are specified as snug-tightened joints in accordance with the specification for Structural Joints Using High-Strength Bolts, June 11, 2020. Installation Inspection requirements for Snug-Tight Bolts (Specification for Structural joints, Section 9.1) is suggested.
- 6. Unless noted otherwise, all bolted connections are designed as bearing type connections with bolt threads not excluded from the
- 7. Any type of suspended or load inducing system(s) is prohibited if zero collateral and zero sprinkler loads are designated on the contract. This would include lights, duct work, piping, and insulation types other than 3" standard duty fiberglass blanket insulation, etc.

BUILDING DESIGN CODES Building Code:	South C	Carolina Buildin	<u> </u>	ode 2021
Hot-rolled version:		AISC 360-1		
Cold-formed version:		<u>AISI S100-1</u>	<u> 16</u>	
GENERAL LOADS				
Dead Load:		2.04 ps	sf	
Roof Collateral Load:		2.00 ps		(Misc.)
Sprinkler Load:		0.00 ps	sf	
Roof Live Load:		20.00 ps		
Tributary Live Load Reduction:		YES		
Rainfall Intensity:		7.27 in	n/hr	(5-minute duration year recurrence)
			5-5	/ear recurrence)
WIND LOAD				
,				

	o year
WIND LOAD	
Wind Load (3—sec gust) Vult:	115_ mph
Vasd:	<u>89</u> mph
V service:	75_ mph
Exposure Factor:	B
Wind Condition:	Enclosed_
Internal Pressure Coefficient :	<u>+/- 0.18</u>
Edge Zone Width:	5.00_ Ft

SNOW LOAD 10.00 psf 10.00 psf 1.00 1.00 1.00 Ground Snow Load: Roof Snow Load : Importance Factor: Exposure Factor: Thermal Factor: Slope Factor:

DEFLECTION CRITERIA Main Frames Horizontal: Roof Panels: Main Frames Vertical: L/180 Purlins: L/180 Bearing Frame Rafter: Wall Panels: L/120 Endwall Columns: Girts: Wind Frame Horizontal: H/60

For components, claddings and MWFRS, deflections involving wind are based on 10 year serviceability wind pressures.

SEISMIC LOAD Risk Category:

II - Normal Seismic Importance Factor 1.0000 0.3211 Structural Response Acceleration (Ss): 0.1053 Structural Response Acceleration(S1): Site Class: 0.3302 Design Spectral Response (Sds): 0.1673 Design Spectral Response (Sd1): Seismic Design Category:

Lateral Longitudinal Framing Direction: 'Structural Steel Systems Not Specifically Structural Syst: Detailed for Seismic Resistance'

Response Modification Factor(s) : Deflection Amplification 3.0 0.1101 esimic Response Coefficient(s) (Cs): 0.1101 Design Base Shear 3.2**0** (Kips) Analysis Procedure uivalent Lateral Force

Profile: Super Span X Gauge: 26 Color: Galvalume Plus UL580 Class 90: Yes Clip Type if Standing Seam:

WALL PANEL

Profile: Super Span X Gauge: <u>26</u> Color: <u>SMP Light Stone</u>

PRIMARY FRAMING

Built-Up & Hot-Rolled: Gray Oxide Primer

SECONDARY FRAMING

Purlins, Eave Struts: Pre-Galvanized Girts, Light Gage Columns: Pre-Galvanized Pre-Galvanized Light Gage Jambs & Headers: N/A Base Angle Finish:

Hot-Dip Galvanizing conforms to the ASTM A123 specification. Pre-Galvanized members conform to the ASTM A653, Grade 50. Coating G-90 specification.

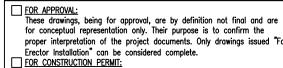
APPROVAL SPECIFICATIONS

- 1. Approval of the Metal Building Provider drawings and/or calculations indicate that the Metal Building Provider has correctly interpreted the contact requirements. This approval constitutes the customer acceptance of the Metal Building Provider design, concepts, assumptions, and loadings.
- 2. Failure to respond to clouded areas and areas to verify may result in additional costs and/or schedule delays for which the Metal Building Provider will not be responsible.
- 3. Any changes made after the Metal Building Provider's customer has signed and returned the Metal Building Provider drawings and/or calculations and the project is released for fabrication shall be billed to the Metal Building Provider customer including material, engineering, and other costs. An additional fee may be charged if the project must be moved in the fabrication and/or the shipping schedule.
- 4. It is the responsibility of the customer to field verify all existing conditions prior to fabrication.
- 5. It is imperative that any changes to these drawings:
- 5.1. Be made in contrasting ink.
- 5.2. Be legible and unambiguous. 5.3. Have all instances of changes clearly indicated.
- 6. A dated signature, in the designated areas, is required on all pages. The signature must be from the person authorized on the
- contract or a person authorized, in writing, by the Metal Building Provider customer. 7. The Metal Building Provider reserves the right to resubmit drawings with extensive or complex changes required to avoid misfabrication.
- This may impact the delivery schedule. 8. Any changes noted on the drawings not in conformance with the terms and requirements of the contract between the Metal Building Provider and its customer are not binding on the Metal Building Provider unless subsequently acknowledged and agreed to in writing by change order or separate documentation.
- 9. Waiving the approval process by designating the order "For Production" supercedes notes 1,2,5,6, and 8 in this section, and constitutes the customer acceptance of the Metal Building Provider's design, concepts, assumptions, and loadings.

DRAWING SCHEDULE

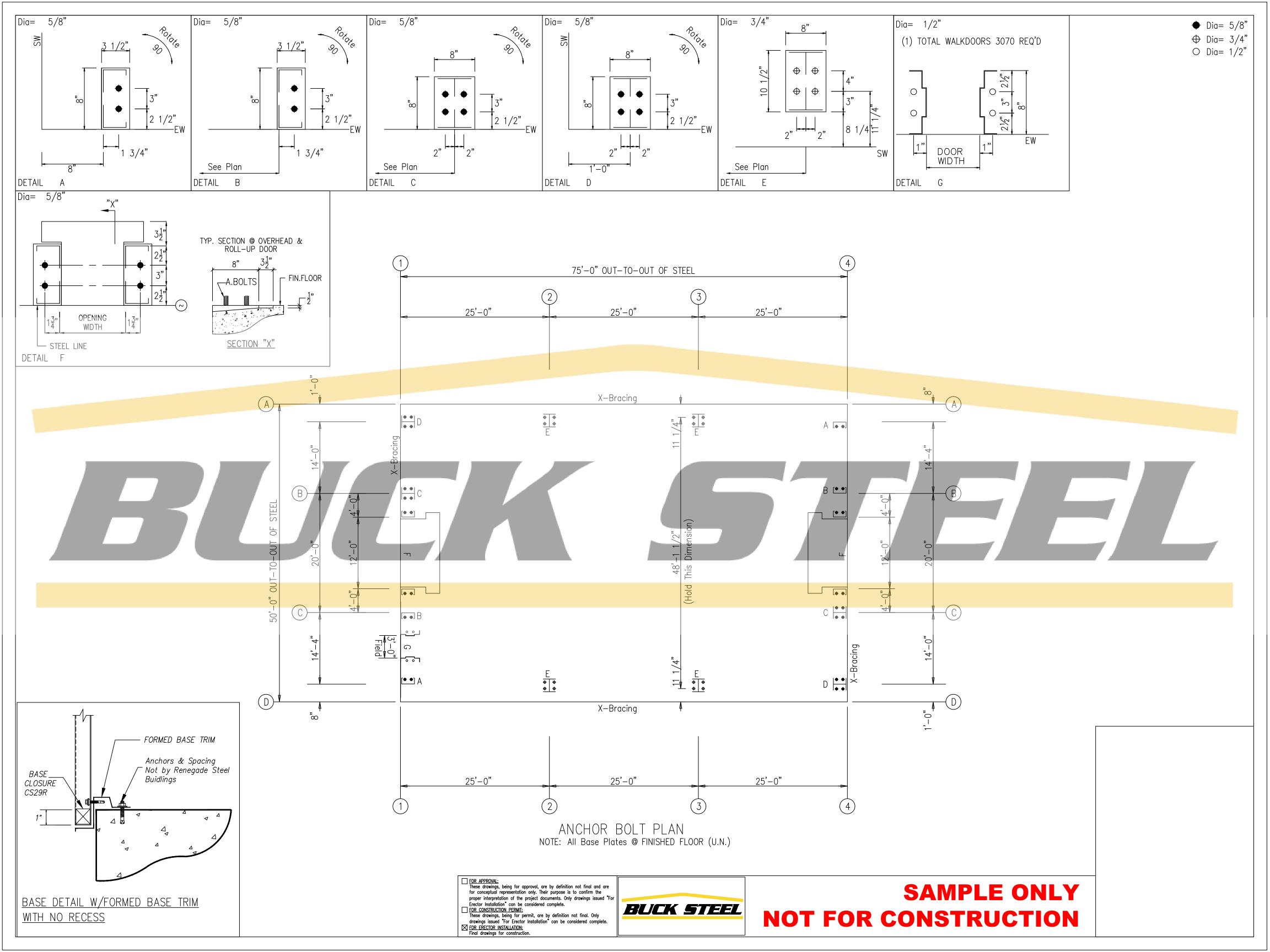
DWG NO.0	ISSUE	DATE	DESCRIPTION
C1	0	08.28.23	COVER SHEET
F1	0	08.28.23	ANCHOR BOLT PLAN & DETAILS
F2	0	08.28.23	ANCHOR BOLT REACTIONS
P1	0	08.28.23	RIGID FRAME ELEVATION
E1	0	08.28.23	ROOF FRAMING PLAN
E2	0	08.28.23	ROOF SHEETING PLAN
E3	0	08.28.23	ENDWALL FRAME & SHEETING ELEVATION
E4	0	08.28.23	ENDWALL FRAME & SHEETING ELEVATION
E5	0	08.28.23	SIDEWALL FRAME & SHEETING ELEVATION
E6	0	08.28.23	SIDEWALL FRAME & SHEETING ELEVATION
E7	0	08.28.23	BUILDING SECTIONS
D1	0	08.28.23	STANDARD DETAILS PAGE
D2	0	08.28.23	STANDARD DETAILS PAGE
D3	0	08.28.23	STANDARD DETAILS PAGE
D4	0	08.28.23	STANDARD DETAILS PAGE

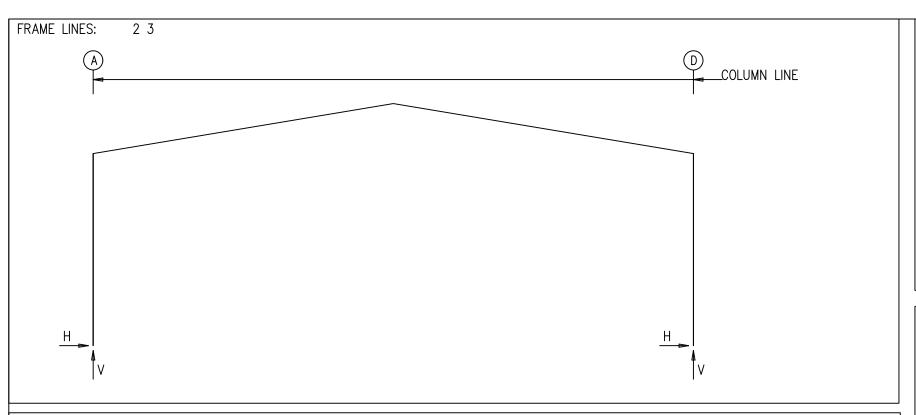




These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete FINAL DESCRIPTION FOR EXECUTION FINAL DESCRIPTION FOR EXECUTION FOR EXEC







ENDW	VALL	COLUN	MN:	BASIC	BASIC COLUMN REACTIONS (k))A/* 1
Frm Line 1 1 1	Col Line A B C D	Dead Vert 0.3 0.8 0.7 0.2	Collat Vert 0.1 0.5 0.5	Live Vert 1.4 4.9 4.9	Snow Vert 0.7 2.5 2.5 0.7	Wind_Le Horz -2.1 0.0 0.0 0.0	eft1 Vert -3.7 -2.0 -2.7 -1.1	Wind_Ri Horz 0.0 2.1 0.0 0.0	ight1 Vert 1.7 -5.5 -4.5 -1.2	Wind_Left2 Horz Ver -1.7 -2.7 0.0 -1.1 0.0 -1.4 0.0 -0.5	7 0.0 1.9 1.7 -3.8 4 0.0 -3.2	Wind Press Horz 0.0 -2.3 -2.3 0.0
Frm Line 1 1 1	Col Line A B C	Wind Suct Horz 0.0 2.3 2.3 0.0	Wind_ Horz 0.0 0.4 0.0 0.0	Long1 Vert -1.1 -4.6 -2.6 -1.0	Wind_Long2 Horz Vert -0.4 -1.5 0.0 -2.1 0.0 -4.1 0.0 -1.6	Seis Horz -0.6 0.0 0.0 0.0	S_Left Vert -0.8 0.8 0.0 0.0	Seis_ Horz 0.0 0.6 0.0 0.0	_Right Vert 0.9 -0.9 0.0 0.0	Seis Long Vert 0.0 0.0 0.0	-MIN_SNOW Horz Vert 0.0 0.7 0.0 2.5 0.0 2.5 0.0 0.7	
Frm Line 1 1 1	Col Line A B C	E1UNB_ Horz 0.0 0.0 0.0 0.0		E1UNB_SL Horz 0.0 0.0 0.0		0.0	0.0	0.0	0.0	0.0	0.0	Wind
Frm Line 4 4 4	Col Line D C B A	Dead Vert 0.3 0.8 0.7 0.2 Wind	Collat Vert 0.1 0.5 0.5 0.1	Live Vert 1.4 4.9 4.9 1.4	Snow Vert 0.7 2.5 2.5 0.7	Wind_Lo Horz -2.1 0.0 0.0 0.0	vert -3.7 -2.0 -2.7 -1.1	Wind_Ri Horz 0.0 2.1 0.0 0.0	ight1 Vert 1.7 -5.5 -4.5 -1.2	Wind_Left2 Horz Ver -1.7 -2.7 0.0 -1.1 0.0 -1.4 0.0 -0.5 Seis	7 0.0 1.9 1.7 -3.8 4 0.0 -3.2	Press Horz 0.0 -2.3 -2.3 0.0
Frm Line 4 4 4	Col Line D C B A	Suct Horz 0.0 2.3 2.3 0.0	Wind_ Horz 0.0 0.4 0.0 0.0	Vert -1.1 -4.6 -2.6 -1.0	Wind_Long2 Horz Vert -0.4 -1.5 0.0 -2.1 0.0 -4.1 0.0 -1.6	Seis Horz -0,6 0.0 0.0 0.0	Left Vert -0.8 0.8 0.0 0.0	Seis_ Horz 0.0 0.6 0.0 0.0	Right Vert 0.9 -0.9 0.0 0.0	Long Vert 0.0 0.0 0.0 0.0	-MIN_SNOW Horz Vert 0.0 0.7 0.0 2.5 0.0 2.5 0.0 0.7	
Frm Line 4 4 4	Col Line D C B	E2UNB_ Horz 0.0 0.0 0.0 0.0	_SL_L- Vert 0.7 2.9 1.2 0.1	0.0 0.0 0.0	R- Vert 0.1 1.2 2.9 0.7							
ENDV	VALL	COLUN	MN:	MAXIMU	JM REACTIONS, A	NCHOR B	OLTS, & B	ASE PLAT	ES			

_	0.1			umn_Reac				5 .	. /. \	5	5		
Frm Line	Col Line	Load Id	Hmax H	V Vmax	Load Id	Hmin H	∨ Vmin 	Qty	t(in) Dia	Base Width	e_Plate(in) Length -	Thick	Elev. (in)
1	Α	3 2	0.0 0.0	-2.0 2.4	3	0.0	-2.0	4	0.625	8.000	8.000	0.375	0.0
1	В	7 1	1.4 0.0	-2.8 6.2	8 7	-1.4 1.4	-2.3 -2.8	4	0.625	8.000	8.000	0.375	0.0
1	С	7 1	1.4 0.0	-2.3 6.2	9 7	-1.4 1.4	-2.0 -2.3	2	0.625	3.500	8.000	0.250	0.0
1	D	6 1	0.0 0.0	-0.8 1.8	6	0.0	-0.8	2	0.625	3.500	8.000	0.250	0.0
4	D	3 2	0.0 0.0	-2.0 2.4	3	0.0	-2.0	4	0.625	8.000	8.000	0.375	0.0
4	С	7 1	1.4 0.0	-2.8 6.2	8 7	-1.4 1.4	-2.3 -2.8	4	0.625	8.000	8.000	0.375	0.0
4	В	7 1	1.4 0.0	-2.3 6.2	9 7	-1.4 1.4	-2.0 -2.3	2	0.625	3.500	8.000	0.250	0.0
4	Α	6 1	0.0 0.0	-0.8 1.8	6	0.0	-0.8	2	0.625	3.500	8.000	0.250	0.0

RIGID	FRAME	_ -:	MAXIM	MAXIMUM REACTIONS, ANCHOR BOLTS, &					ATES				
Column_Reactions(k)				D-I	ı/:\	D	DI-1-(:-)		Π				
Frm Line	Col Line	Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Qty	t(in) Dia	Width	e_Plate(in) Length	Thick	Elev. (in)
2*	Α	1	4.4	10.6	3 5	-3.7 -0.1	-5.6 -5.9	4	0.750	8.000	10.50	0.375	0.0
2*	D	4 1	3.7 -4.4	-5.6 10.6	1 6	-4.4 0.1	10.6 -5.9	4	0.750	8.000	10.50	0.375	0.0
2*	Frame lin	nes:	2 3										

RIGID FRAME: BASIC COLUMN REACTIONS (k) Frame Column -----Dead------Collateral-----Live-----Snow-----Wind_Left1-Horiz Horiz Vert Horiz Horiz Vert Line Line Vert 7.5 7.5 0.5 -0.5 3.2 -3.2 2.6 -2.6 0.7 1.9 -6.8 -11.20.5 -7.16.3 2* 1.9 6.2 -0.56.8 -11.2-0.7--Wind_Long1-Horiz Vert --Wind_Long2--Seismic_Left Frame Column --Wind_Left2--Wind_Right2-Seismic_Right Horiz Vert Line Line Horiz Vert Horiz Vert Horiz Vert Horiz -6.6 -2.5 -6.6 -0.9 1.5 -11.7 -10.1 -1.5 0.9 -6.70.5 -10.1-0.5-0.30.5 0.3 0.5 -0.5-2.56.7 -11.7 -0.3-MIN_SNOW--F1UNB_SL_L-F1UNB_SL_R-Frame Column -Seismic_Long Horiz Vert Line Line Horiz Vert Horiz Vert Horiz 2.2 0.0 -0.92.6 6.2 2.2 6.0 0.0 -0.9-2.66.2 -2.23.5 Frame lines: 2 3

NOTES FOR REACTIONS

1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.

2.50 3.00

2.00

- 2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- 3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- 4. Loading conditions are:

Endwall

Frame

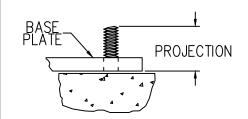
Walk door

- Dead+Collateral+Live
 Dead+Collateral+0.75Live+0.45Wind_Right2
 0.6Dead+0.6Wind_Left1
- 4 0.6Dead+0.6Wind_Right1
- 5 0.6Dead+0.6Wind_kighti
- 6 0.6Dead+0.6Wind_Long2L
- 9 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L

GENERAL NOTES

- 1. All anchor bolts (by others) to have nuts and flat washers.
- 2. All anchor bolts are designed to full S.A.E. diameters with cut threads. No substitutions are allowed.
- 3. The Metal Building Provider is not responsible for the design, materials and workmanship of the foundation. Anchor bolt plans prepared by the Metal Building Provider are intended to show only location, diameter, and projection of anchor bolts required to attach the Metal Building System to the foundation. The Metal Building Provider is responsible for providing to the Builder the loads imposed by the Metal Building System on the foundation. It is the responsibility of the End Customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and/or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. This is typically the responsibility of the Design Professional or Engineer of Record, which is another reason that their involvement in the Construction Project from the outset is highly recommended. (2012 MBMA Metal Building Systems Manual, Section 3.2.2)
- 4. The projection is based from the bottom of the base plate. Adjustments must be made for grout and/or leveling plates.

THREADED ANCHOR BOLT



NOTE: PROJECTION BASED
FROM BOTTOM OF BASE
PLATE. ADJUSTMENTS
SHOULD BE MADE FOR
GROUT AND/OR LEVELING
PLATES.

ANCHOR	BOLT	SUMMA	RY (GRADE	36)
Qty L	ocate	Dia (in)	Туре	Proj (in)	

5/8" 5/8" 3/4"

1/2"

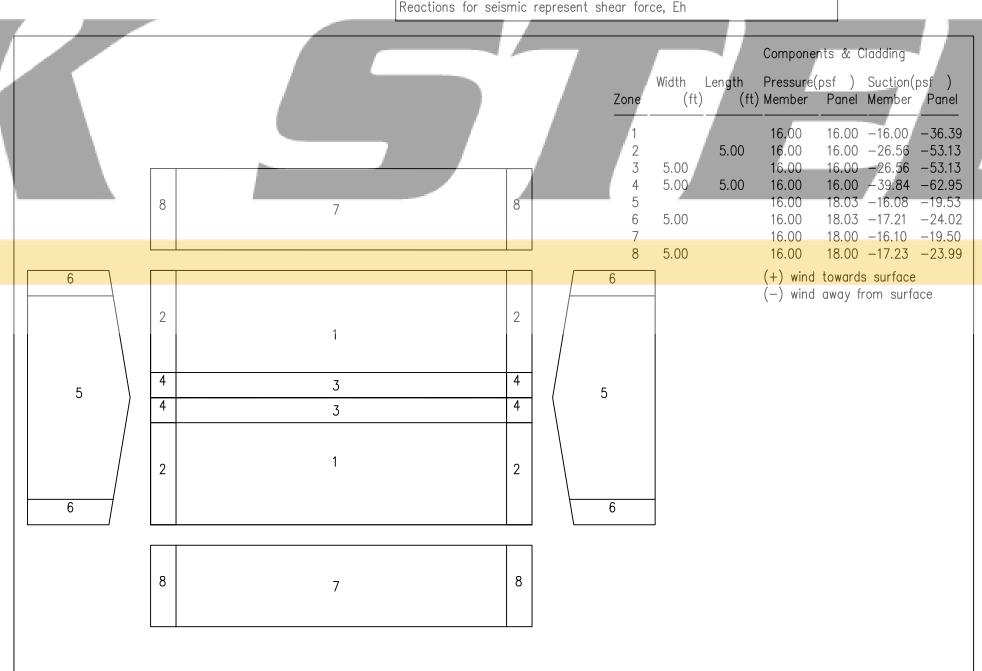
F1554

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			±	Reacti	ions(k)		Panel_	Shear
Wo	III —	- Col	——W	ind —	– — Sei:	smic —	· (lb/	/ft)
Loc	Line	Line	Horz	Vert	Horz	Vert	Wind	Śeis
L_EW	1	A,B	2.1	2.4	0.6	0.8		
F_SW	Ď	2,3	3.6	2.0	1.6	0.9		
R_EW	4	D,C	2.1	2.4	0.6	0.8		
B_SW	Α	3,2	3.6	2.0	1.6	0.9		

BUILDING BRACING REACTIONS



FOR APPROVAL:

These drawings, being for approval, are by definition not final and are for conceptual representation only. Their purpose is to confirm the proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered complete.

☐ FOR CONSTRUCTION PERMIT:

These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.

☐ FOR ERECTOR INSTALLATION:
Final drawings for construction.



SPLICE PLATE & BOLT TABLE Qty Top Bot Int Type Dia Length Thick Length Width 1/2" 4 4 0 A325 3/4" 2" 1'-11"

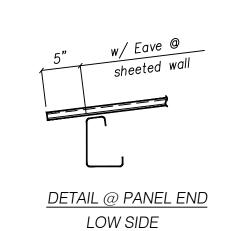
FLANGE BRACE TABLE

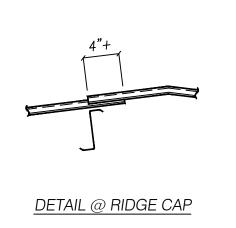
A=L2x2x14GA B=L2x2x12GA C=L2x2x1/8 D=L3x3x3/16

FRAME LINE: 2 3

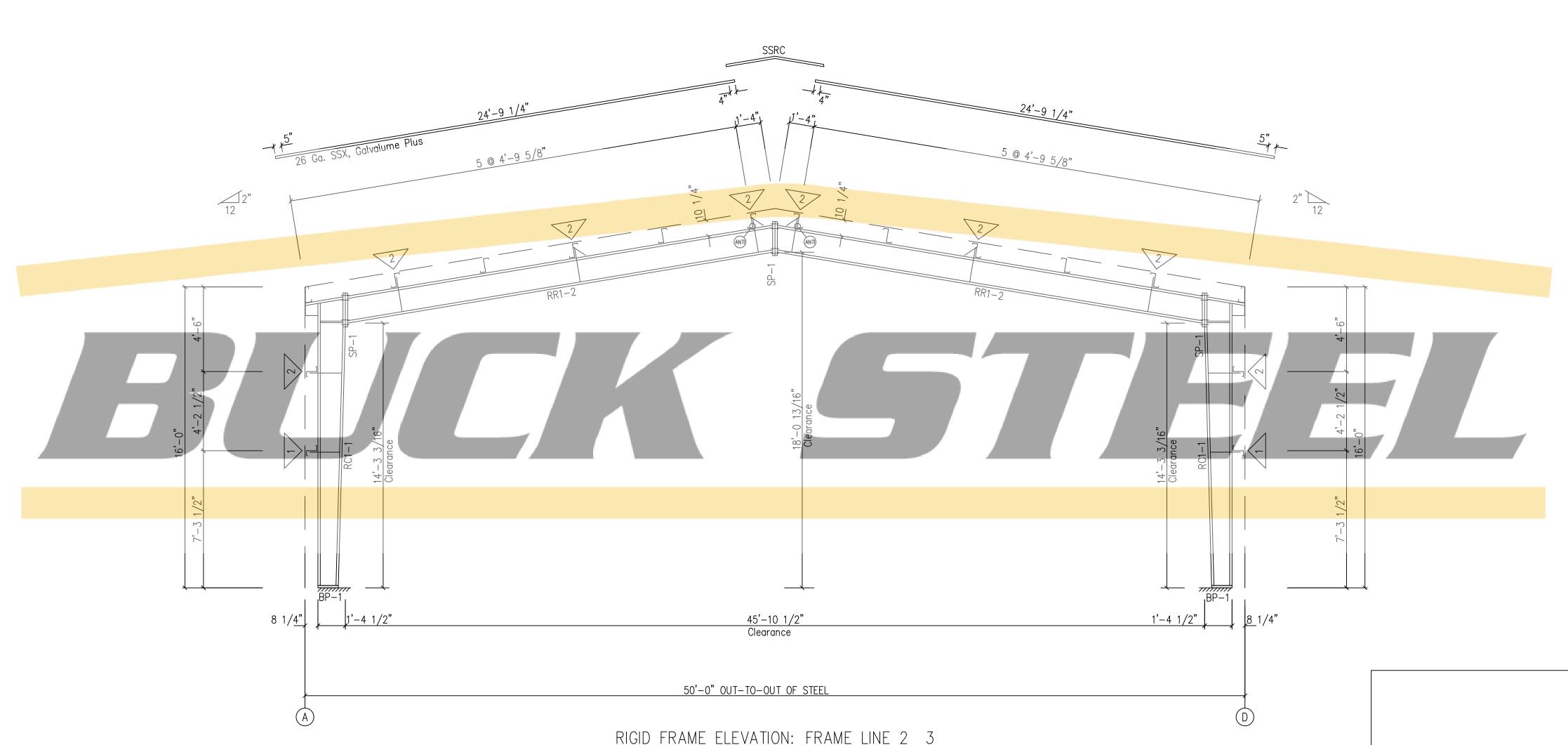
∇ID	# SIDES	MARK	LENGTH	OFFSET	DETAIL	CLIP
1 2	1 1	FB2A FB3A	2'-7" 2'-8"	2'-4" 2'-4"		

BASE PLATE TABLE							
Col							
Mark	Width	Thick	Length				
BP-1	8"	3/8"	10 1/2"				





MEMBER TA	BLE			
	Web Depth	Web Plate	Outside Flange	Inside Flange
Mark	Start/End	Thick		W x Thk
RC1-1	10.0/16.0	0.135	5 x 1/4"	5 x 1/4"
	16.0 / 16.0	0.164	6 x 1/4"	'
RR1-2	14.0′/14.0	0.135	5 x 1′/4"	5 x 1/4"
	14.0/14.0	0.135	5 x 1/4"	5 x 1/4"



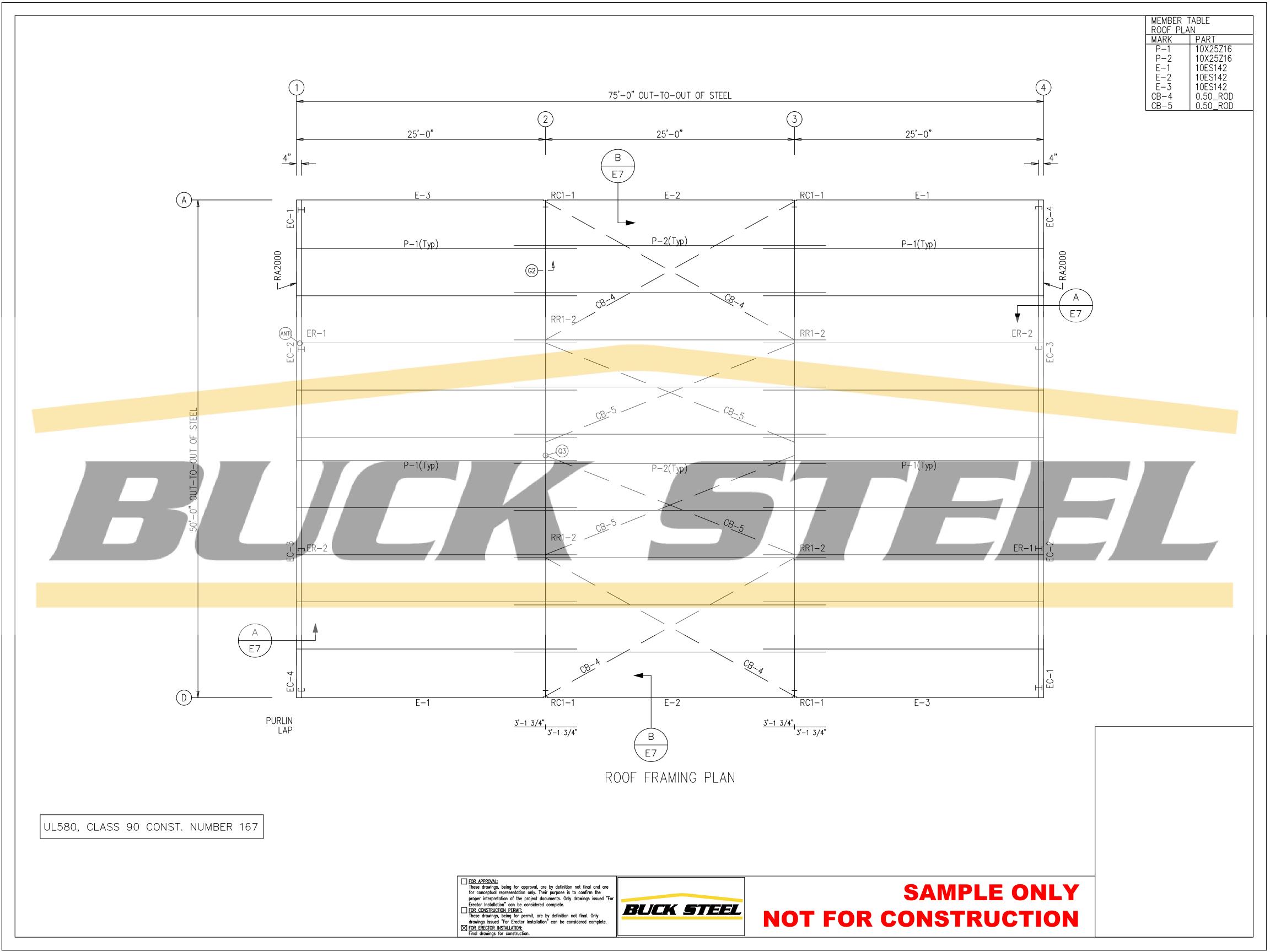
BOLT TIGHTENING (Snug-Tight)

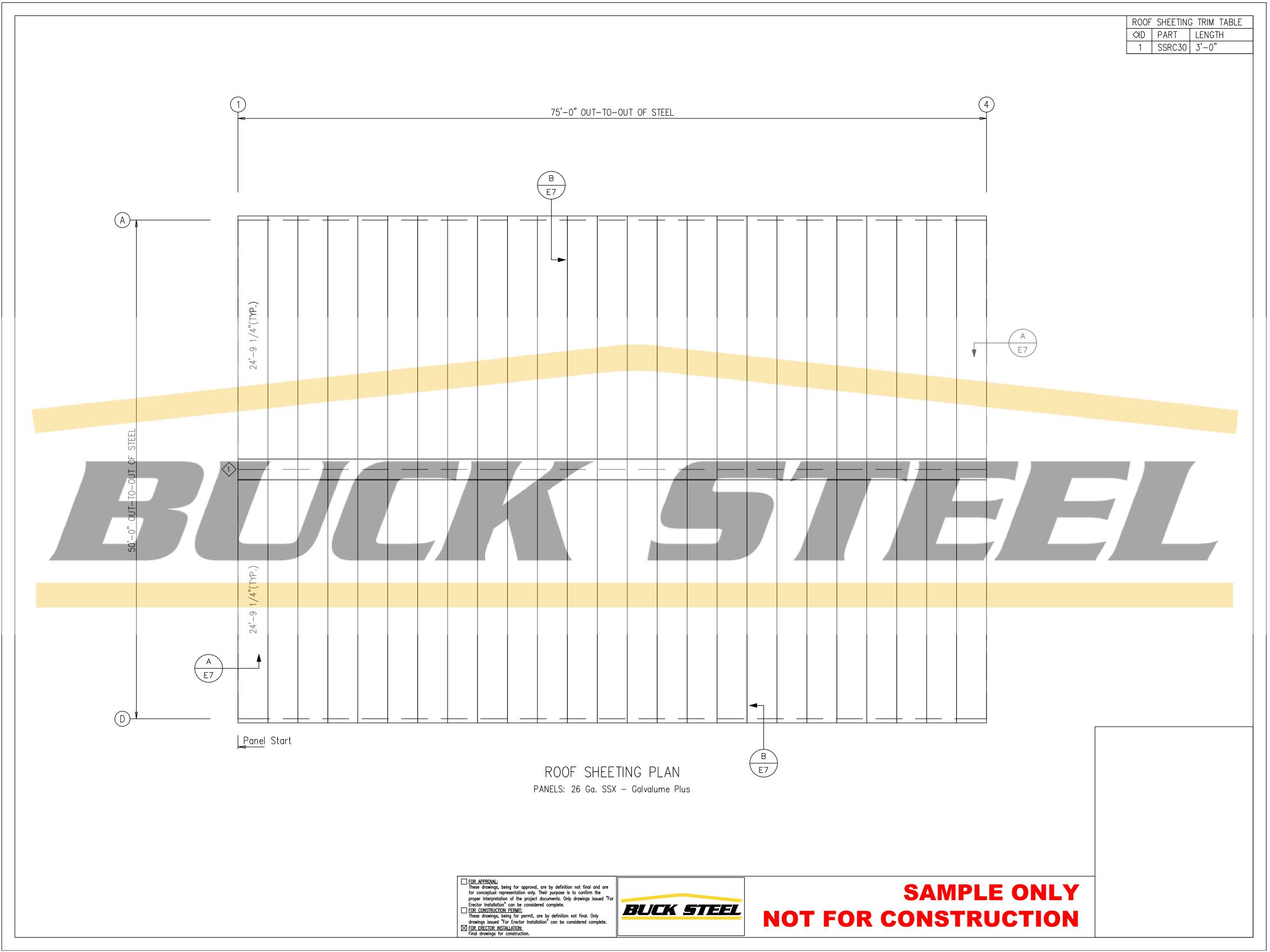
All bolted joints with ASTM F3125 Grade A325 bolts are specified as Snug-Tightened Joints in accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, installation as given in Section 7.1 Washers are not required for Snug-Tightened Joints using standard standard size holes per Section 6.1 of the Specification

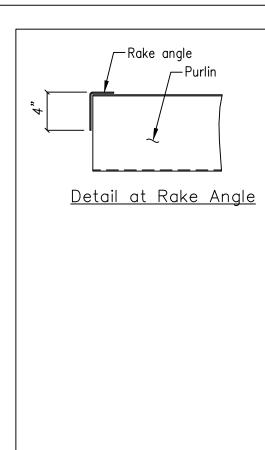
Pretensioning methods, including Turn-of-Nut, calibrated wrench, twist-off tension control bolts or direct tension indicator are not required. Installation inspection requirements for Snug-Tight Bolt is found in Section 9.1 of the Specification.

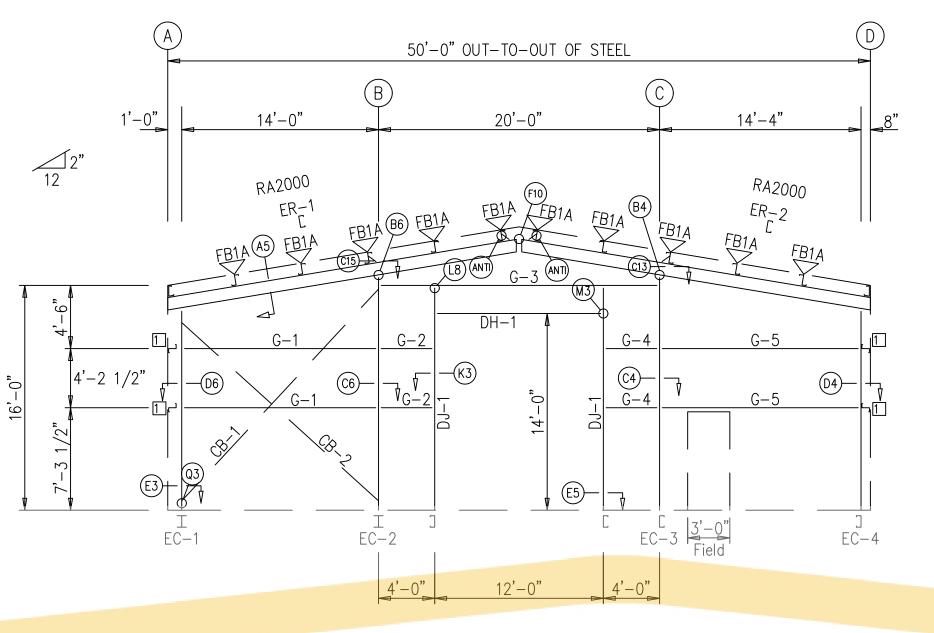
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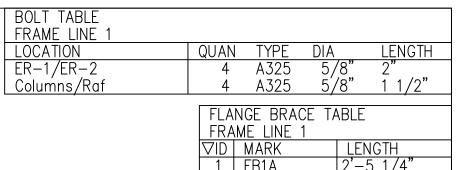








ENDWALL FRAMING: FRAME LINE 1



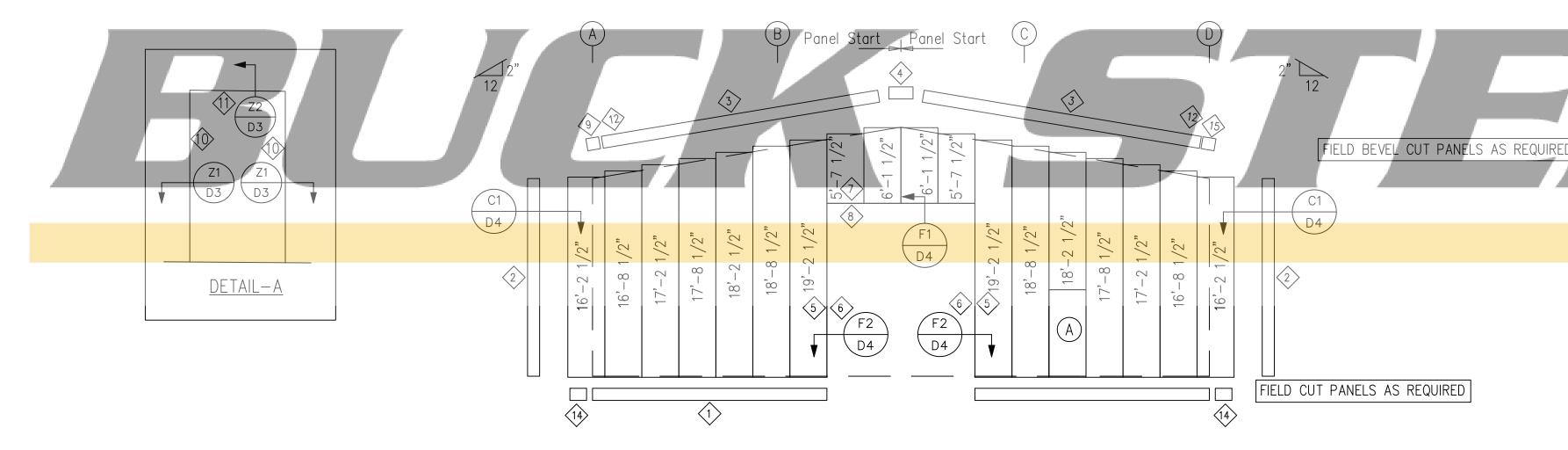
I LRI	Α [2-5 1/4
1	TABLE — T IE LINE —1	HIS WALL ONL
⟨AD	PART	LENGTH
1 2 3 4 5 6 7 8 9 10 11 12 14 15	44BT20 CT-102 FL-16 FL-16B MT-116B FL-22 MT-116B HT-101 FL-16CL FL-22 HT-101 FL-16A 44BAC FL-16CR	20'-3" 16'-4" 15'-3" 14'-4" 14'-4" 12'-4" 12'-4" 7'-4" 3'-4"

CONNECTION PLATES
FRAME LINE 1

DID | MARK/PART

MEMBER TABLE

	FRAME LINE 1	
	MARK	PART
	EC-1	W8X10
	EC-2	W8X10
	EC-3	8M35C12
	EC-4	8M35C14
	ER-1	8M35C12
	ER-2	8M35C12
	DJ-1	8M35C14
	DH-1	_8M25C14
	G-1	8X25Z16
1	G-2	8X25Z16
	G-3	8X25Z14
	G-4	8X25Z16
	G-5	8X25Z16
	CB-1	0.50_ROD
	CB-2	0.50_ROD



GENERAL SHEETING & TRIM NOTES

- 1. Refer to erection drawings for rake angle locations.
- 2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown. 3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
- 4. Roof stitch screws are located at each member with two between members (20" max. spacing). 5. Wall stitch screws are located at each member with one between members (20" max. spacing).
- 6. Skylight stitch screws are at 6" o.c. 7. Start endwall panels at centerline of bldg. unless noted.
- 8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
- 9. Field cut or lap panels as required to fit.
- 10. Field cut panels for all openings.
- 11. Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.
- 12. Gutter support strap spacing: Super Span 3'-0, Super Seam 4'-0, Weather Lok-16 2'-8". 13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
- 14. Downspout straps are located 6" from base and at every girt location. 15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
- 16. Metal shavings must be swept from the roof each day to avoid surface rusting.
- 17. Windows and louvers must be installed before sheeting the walls.
- 18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. SSX — SMP Light Stone

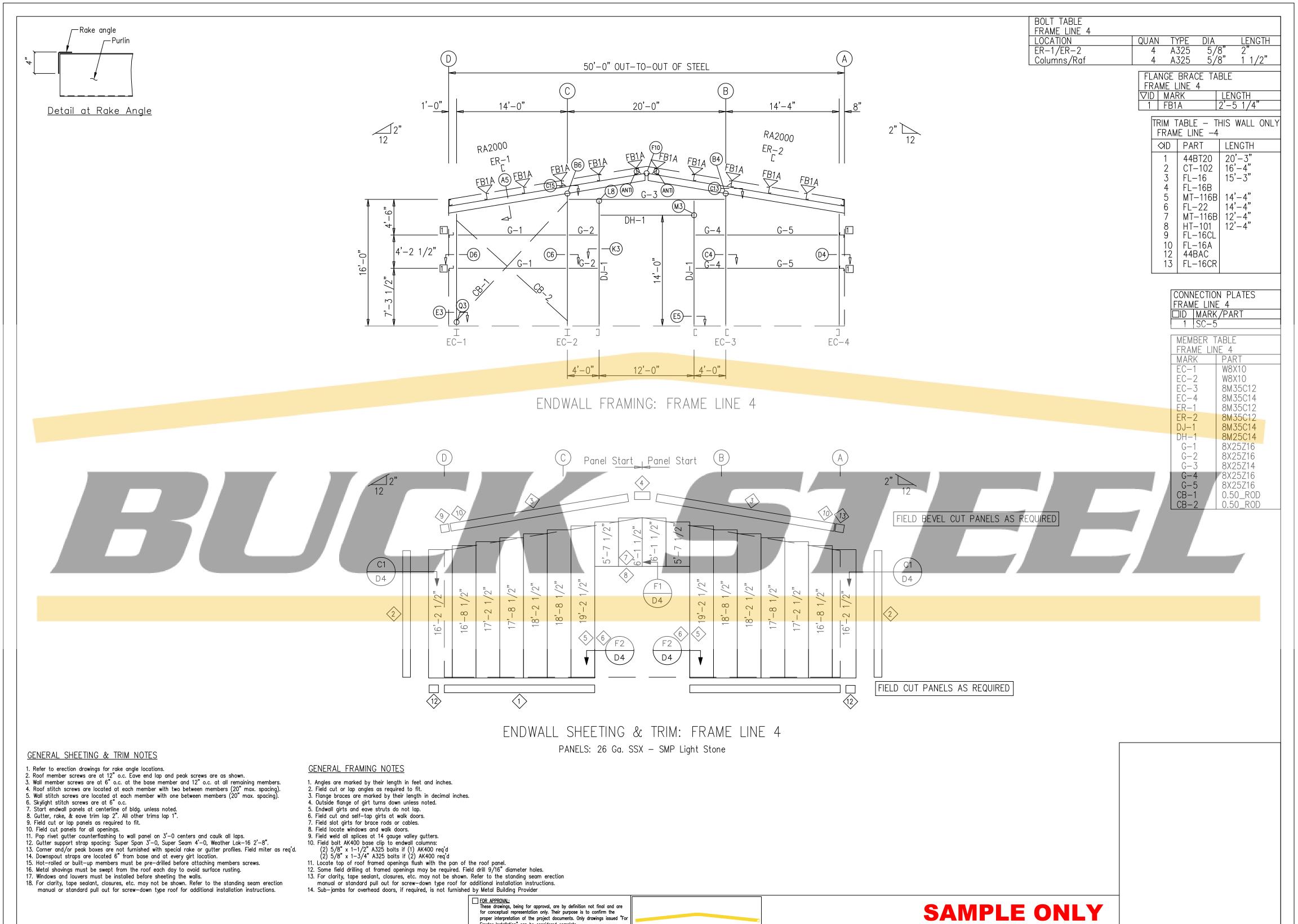
GENERAL FRAMING NOTES

- 1. Angles are marked by their length in feet and inches. 2. Field cut or lap angles as required to fit.
- 5. Flange braces are marked by their length in decimal inches.
- 4. Outside flange of girt turns down unless noted. 5. Endwall girts and eave struts do not lap.
- 6. Field cut and self-tap girts at walk doors.
- 7. Field slot girts for brace rods or cables. 8. Field locate windows and walk doors.
- 9. Field weld all splices at 14 gauge valley gutters.
- 10. Field bolt AK400 base clip to endwall columns:
- (2) 5/8" x 1-1/2" A325 bolts if (1) AK400 req'd (2) 5/8" x 1-3/4" A325 bolts if (2) AK400 req'd
- 11. Locate top of roof framed openings flush with the pan of the roof panel.
- 12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes. 13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection
- manual or standard pull out for screw-down type roof for additional installation instructions. 14. Sub-jambs for overhead doors, if required, is not furnished by Metal Building Provider

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 Final drawings for construction



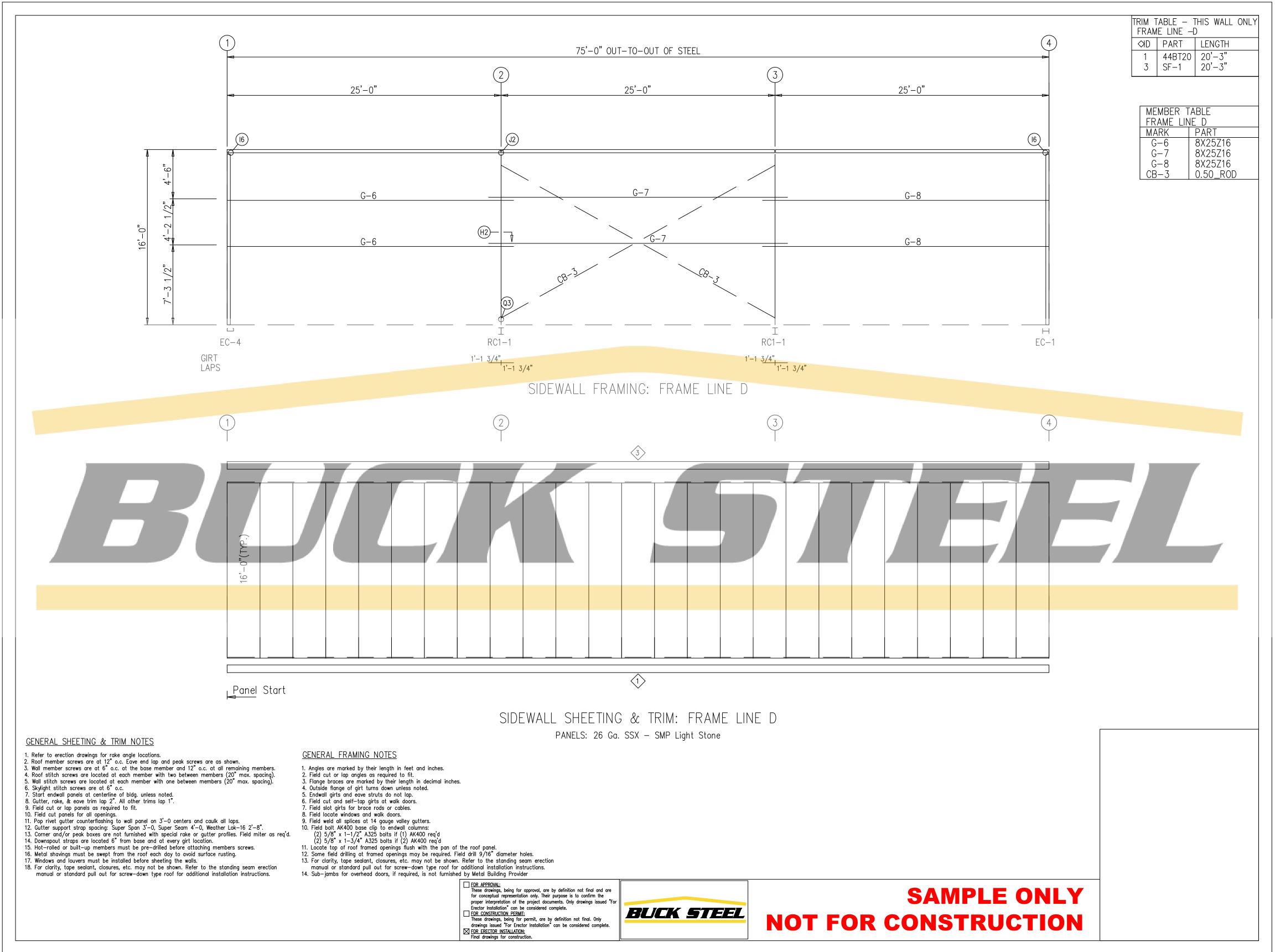


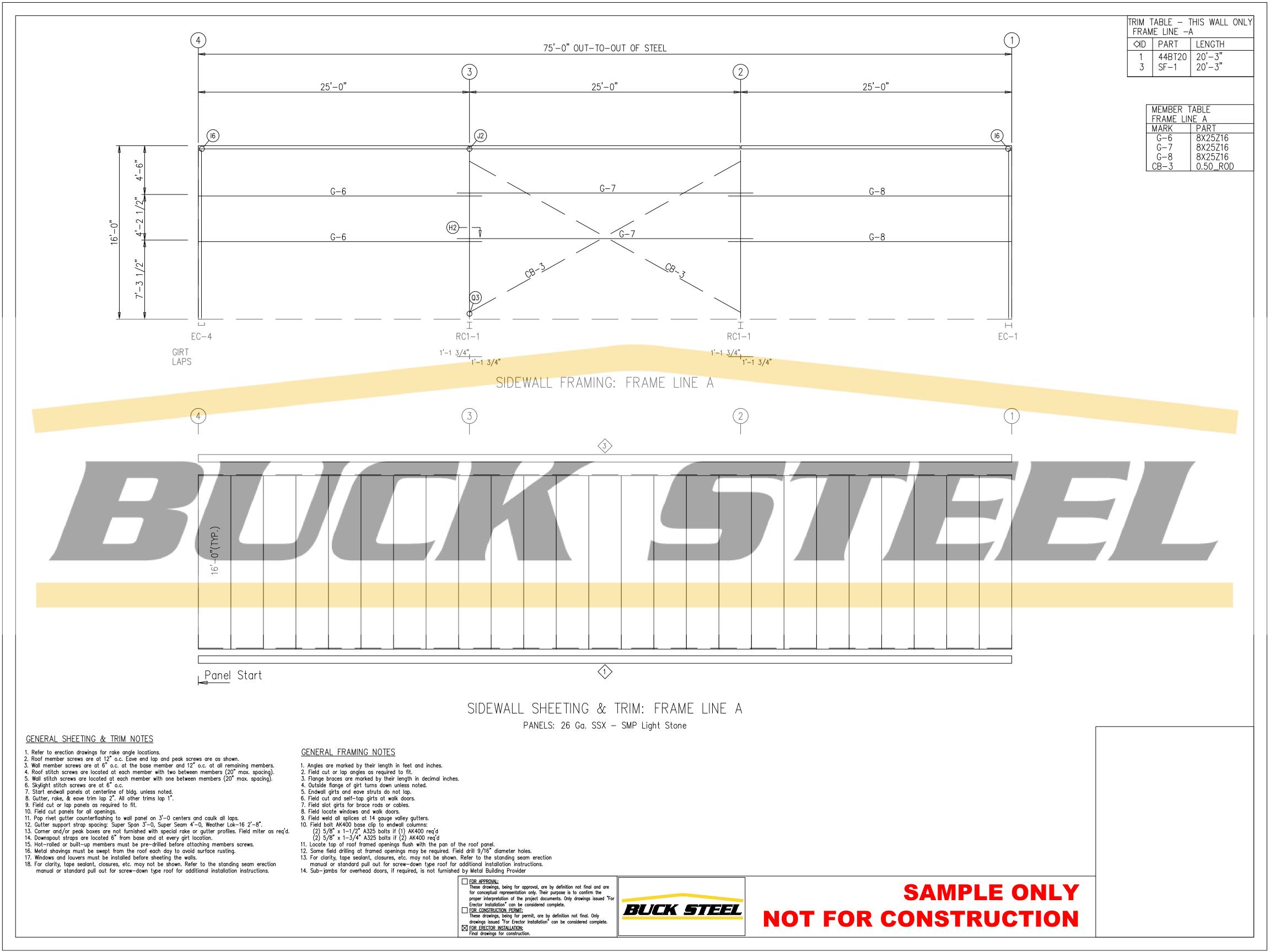
BUCK STEEL

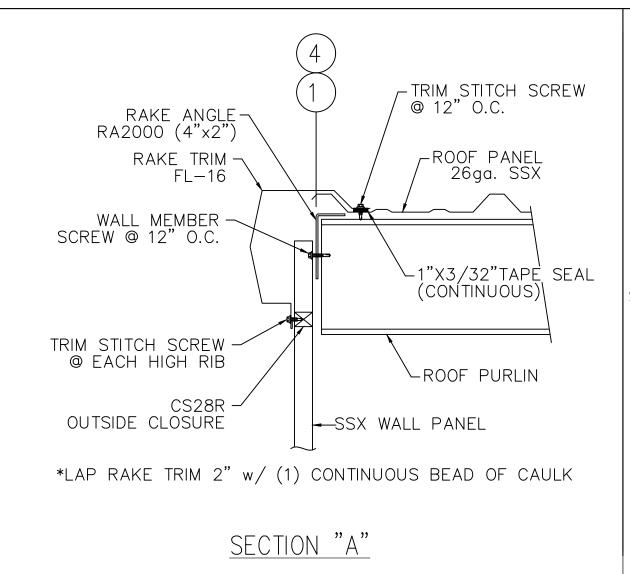
Erector Installation" can be considered complete.

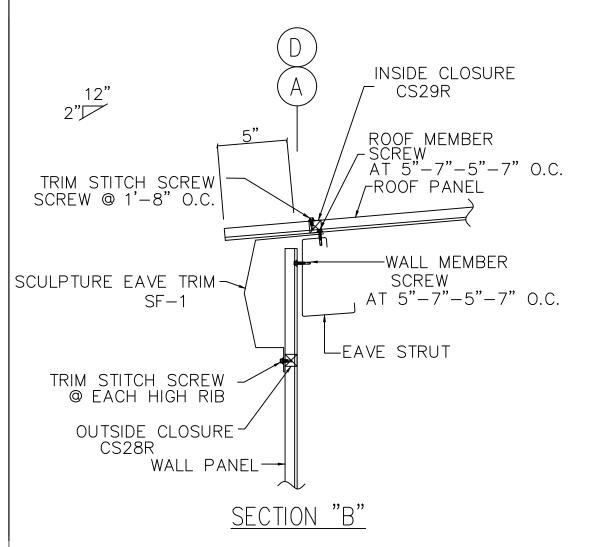
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Final drawings for construction

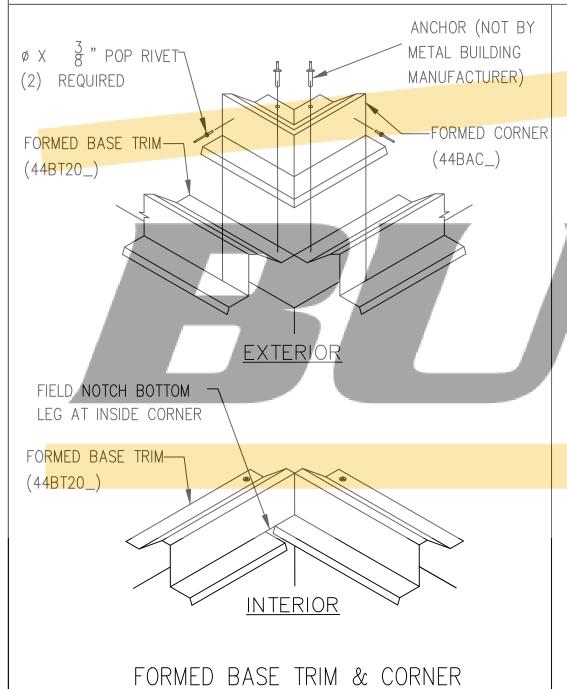
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(44BT20_ & 44BAC_)



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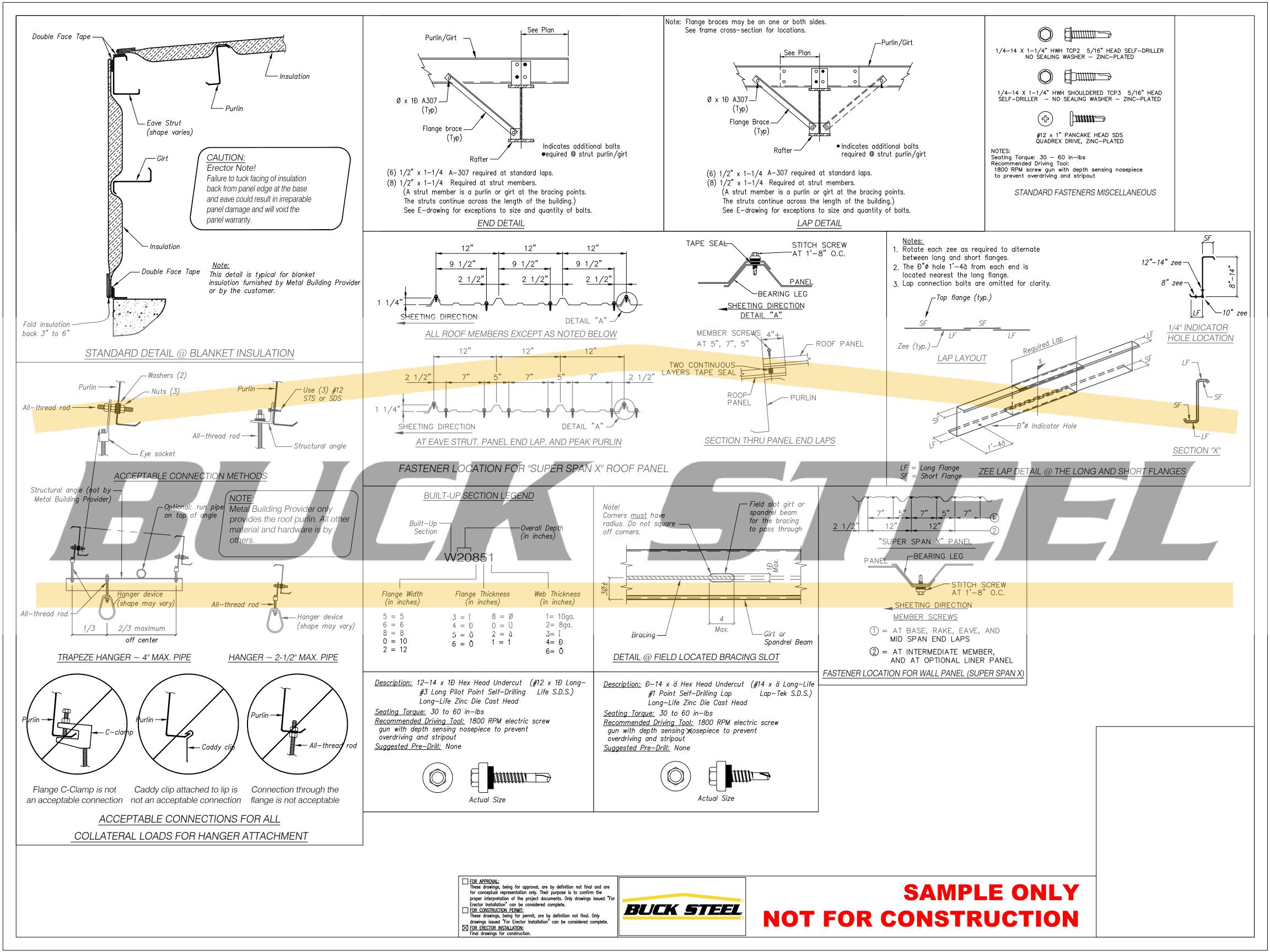
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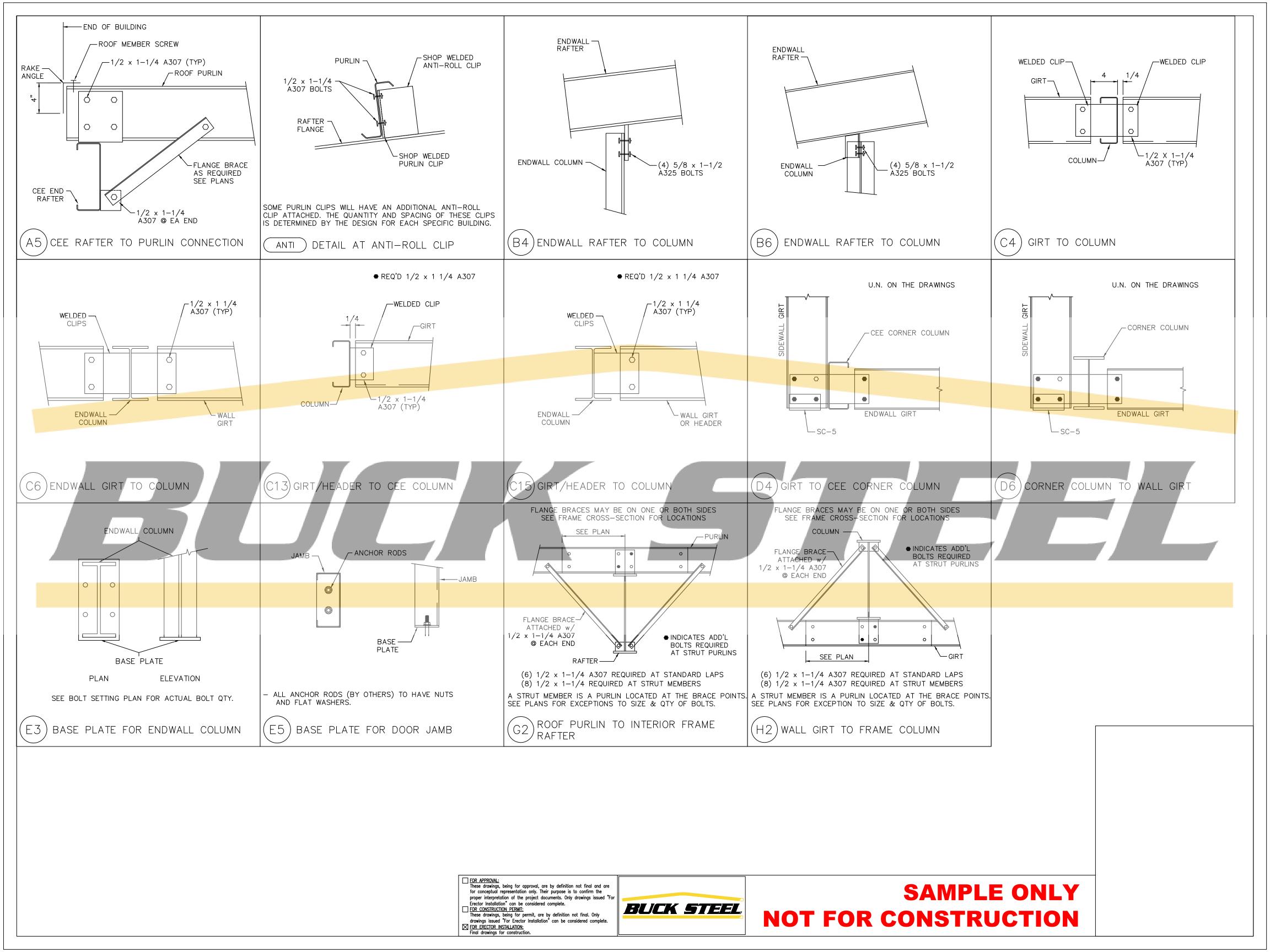
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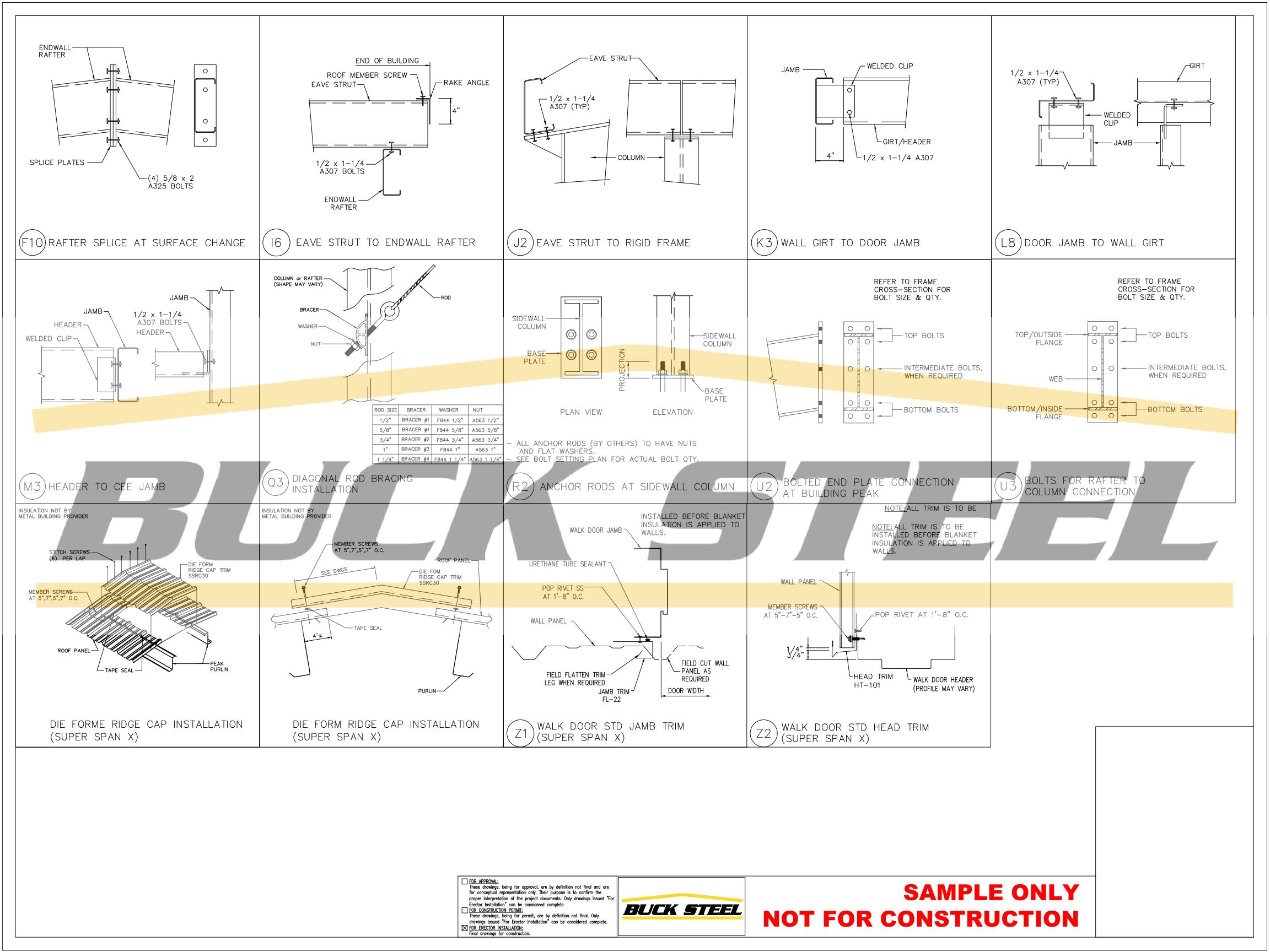
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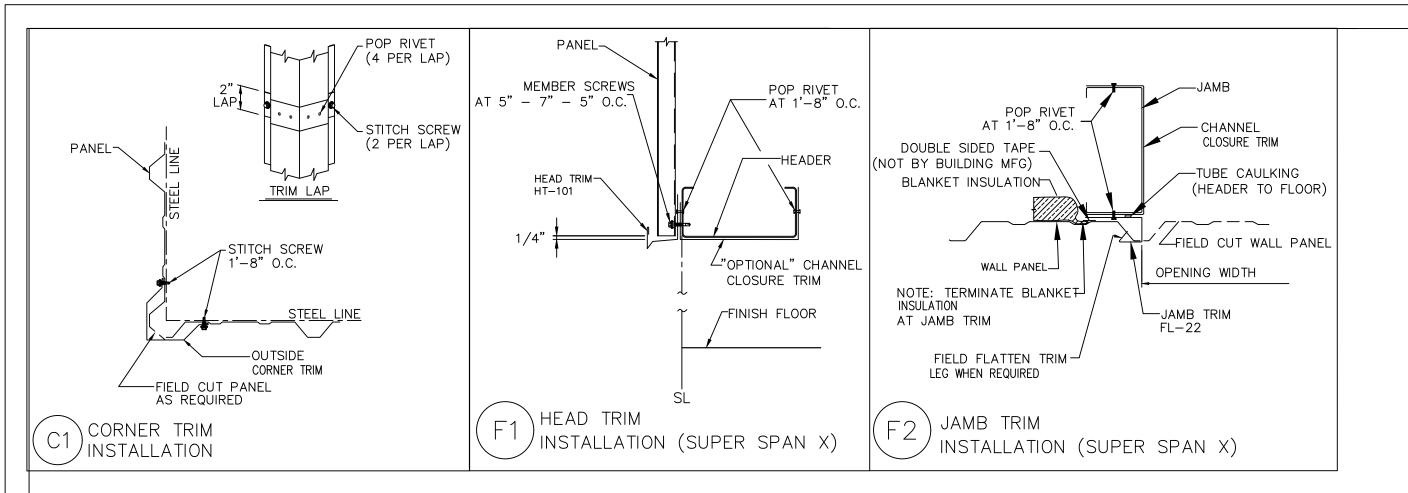
FINAL drawings for construction.











BUCK 577EL

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FOR ERECTOR INSTALLATION:
Final drawings for construction.

